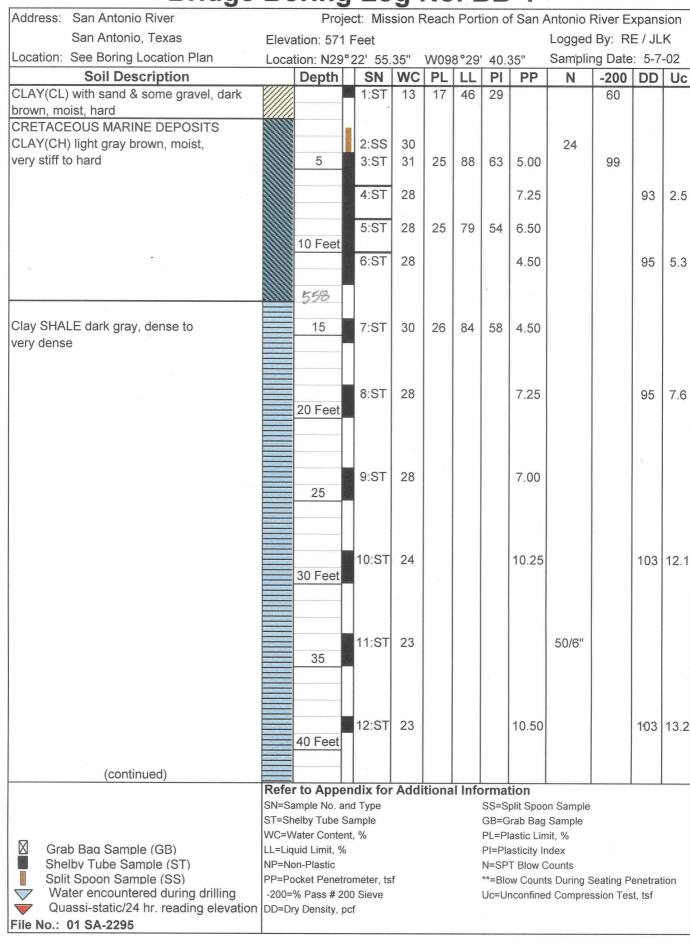
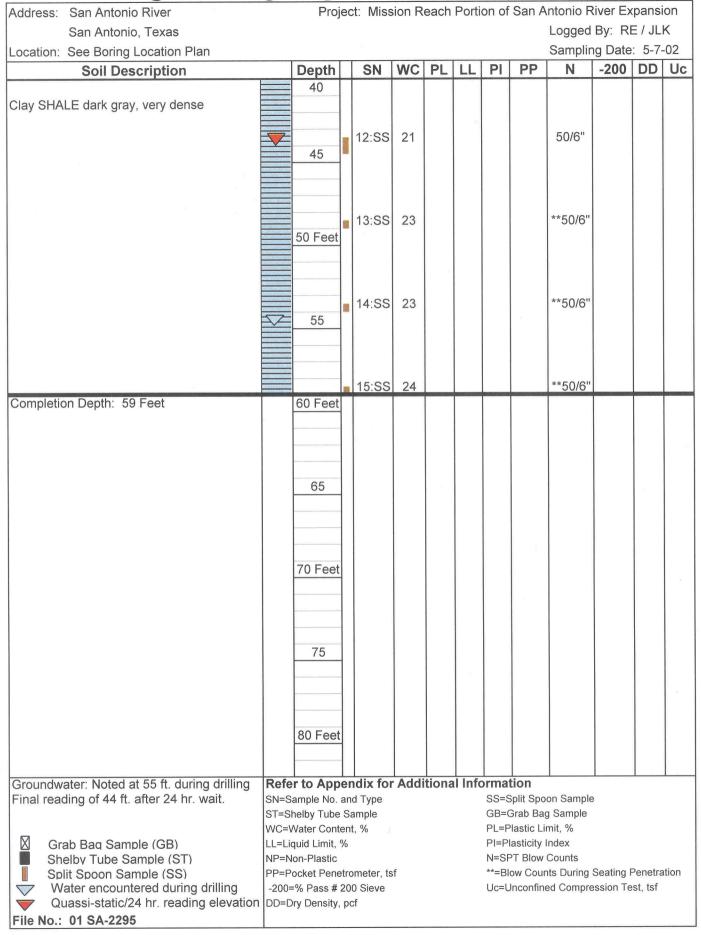
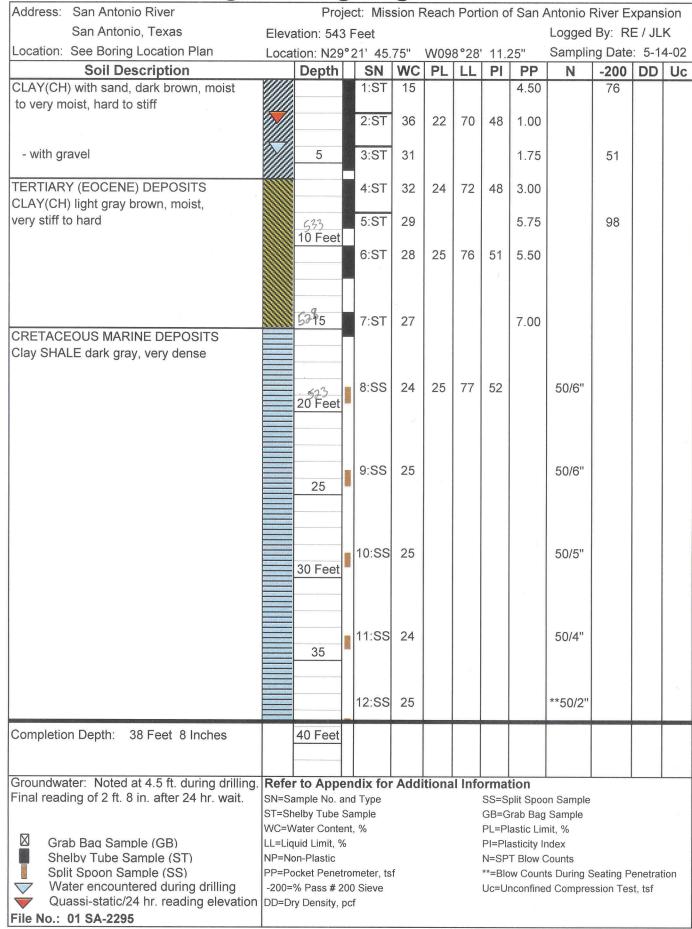
Bridge Boring Log No. BB-1

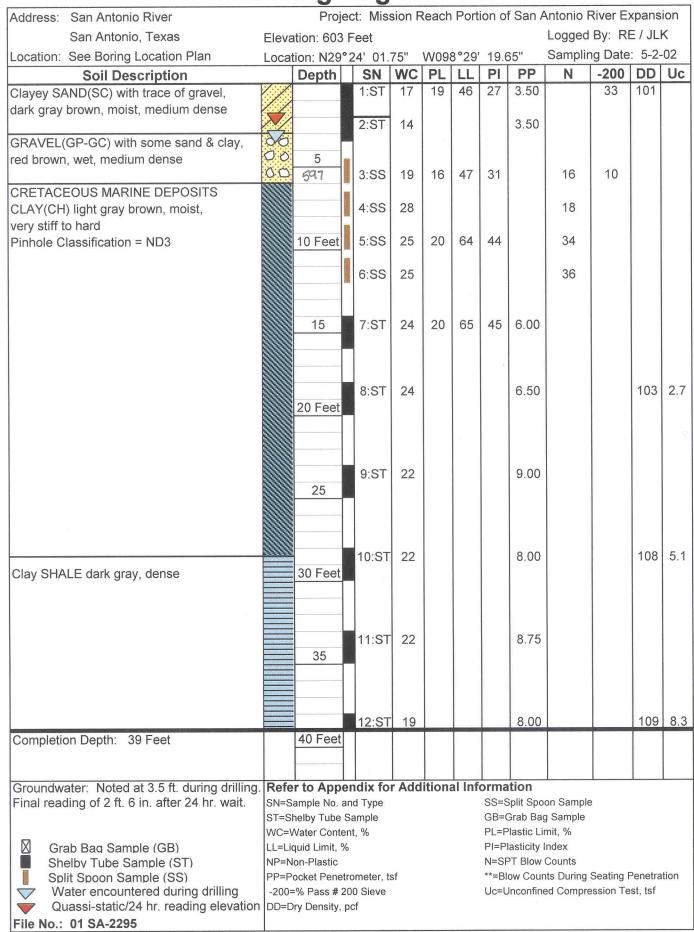


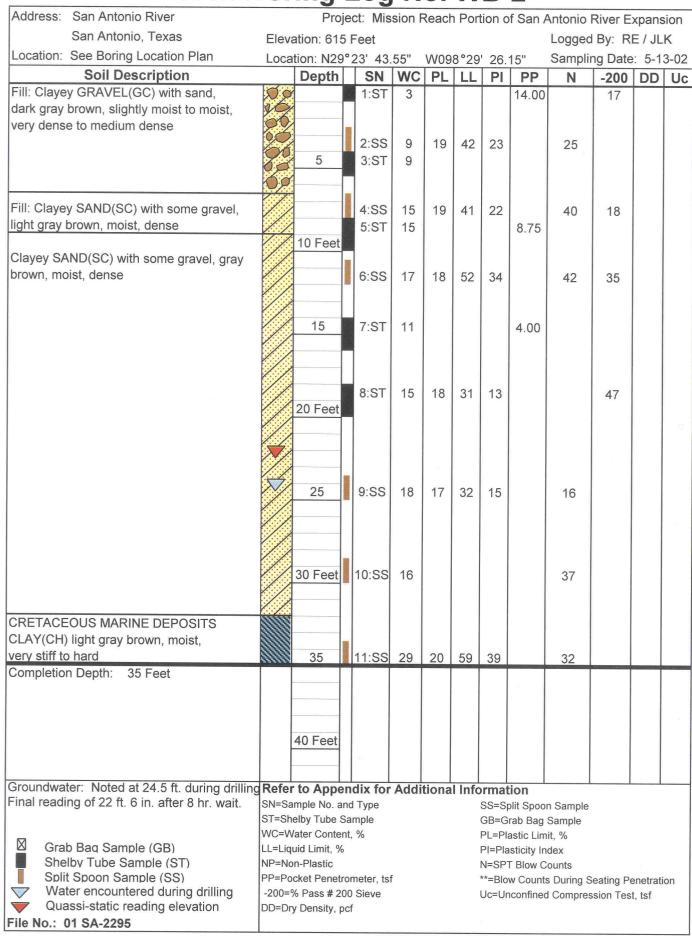
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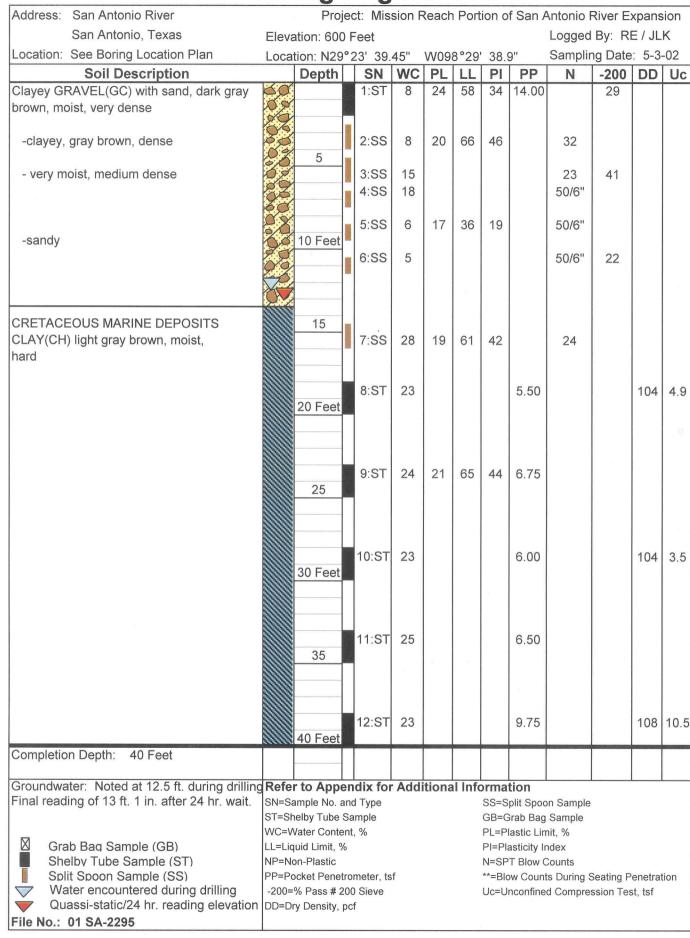


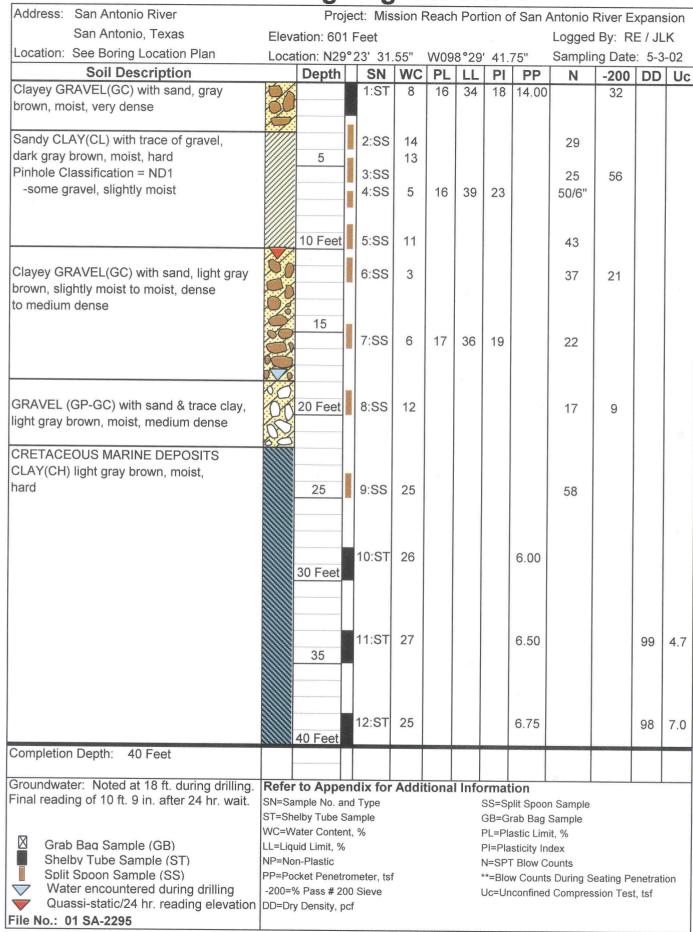
Bridge Boring Log No. BB-2

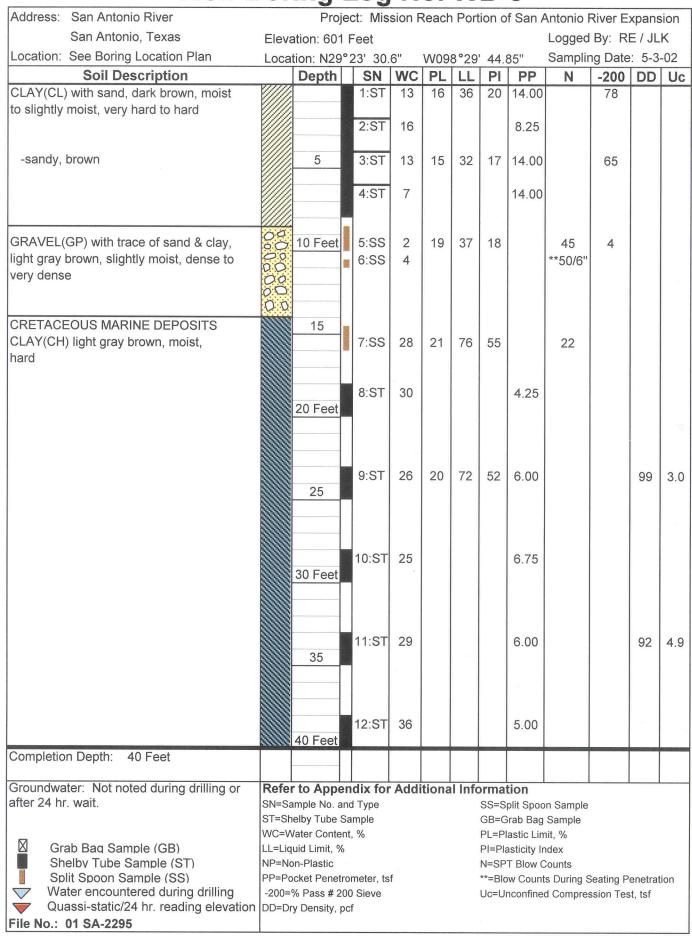


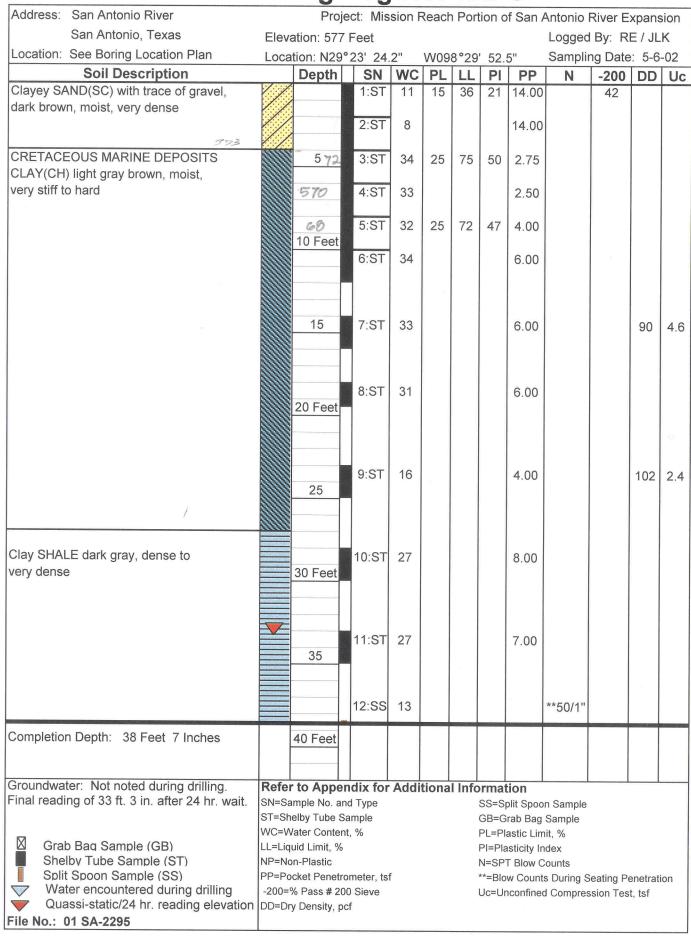


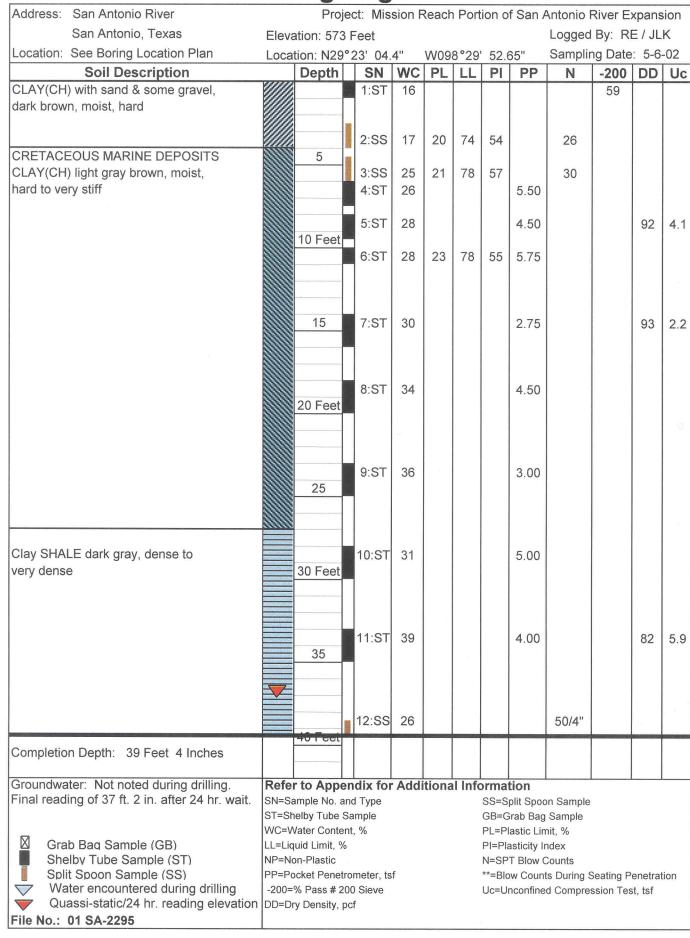


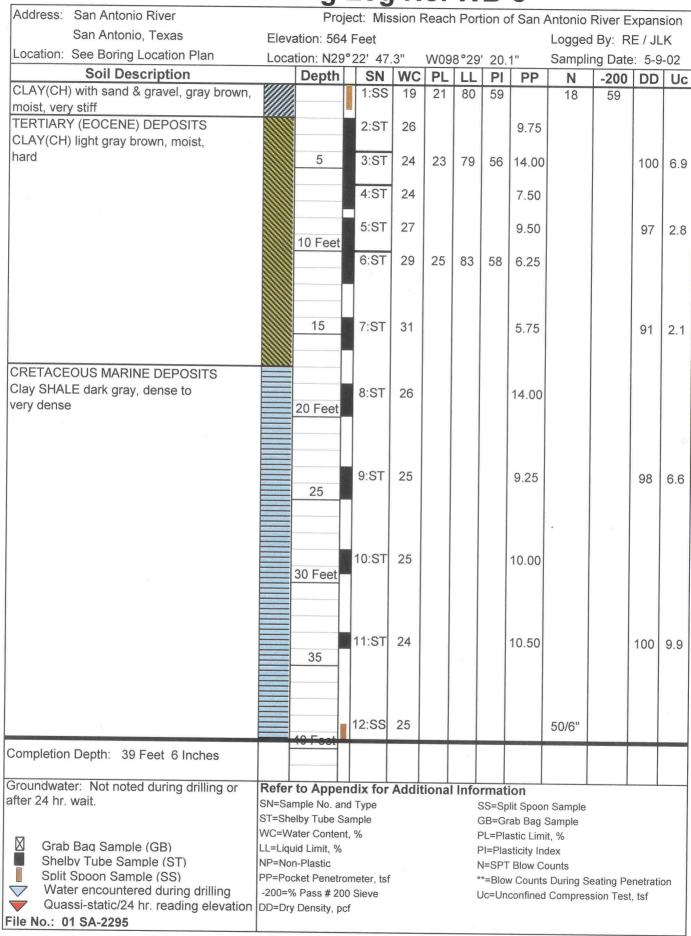


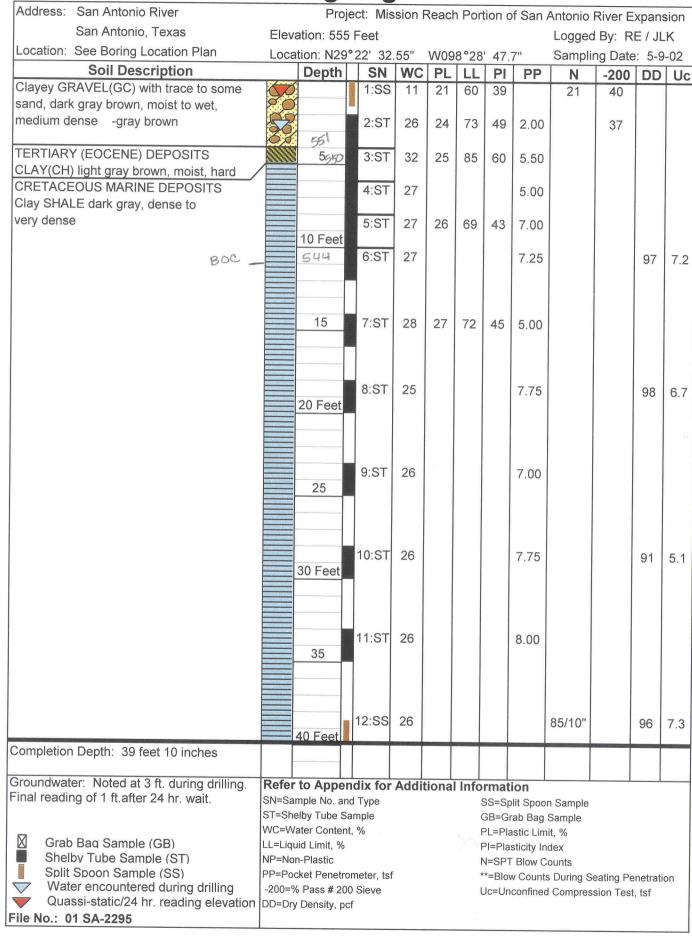


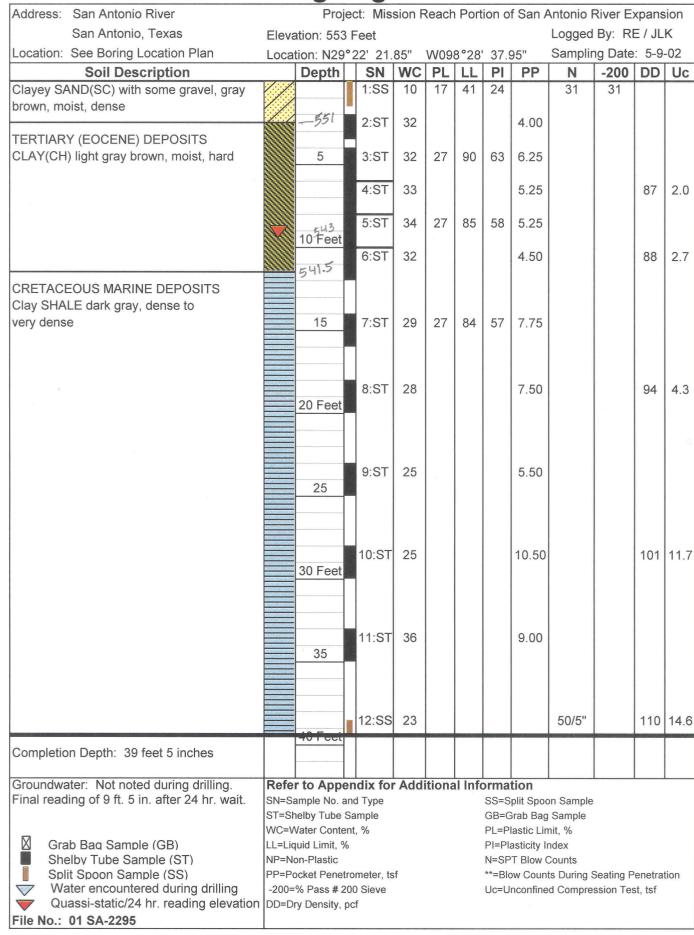


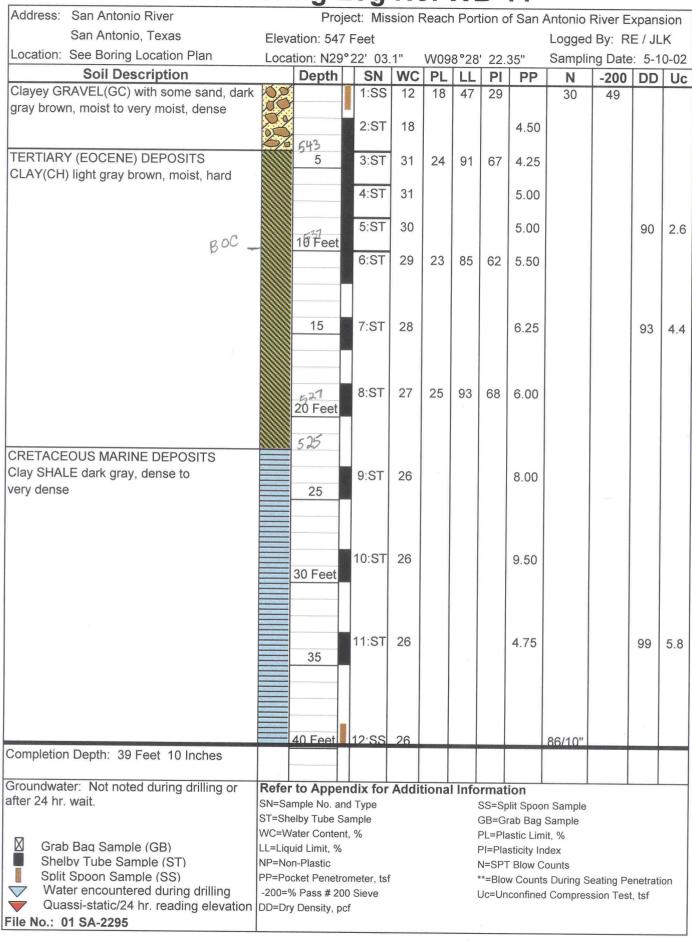


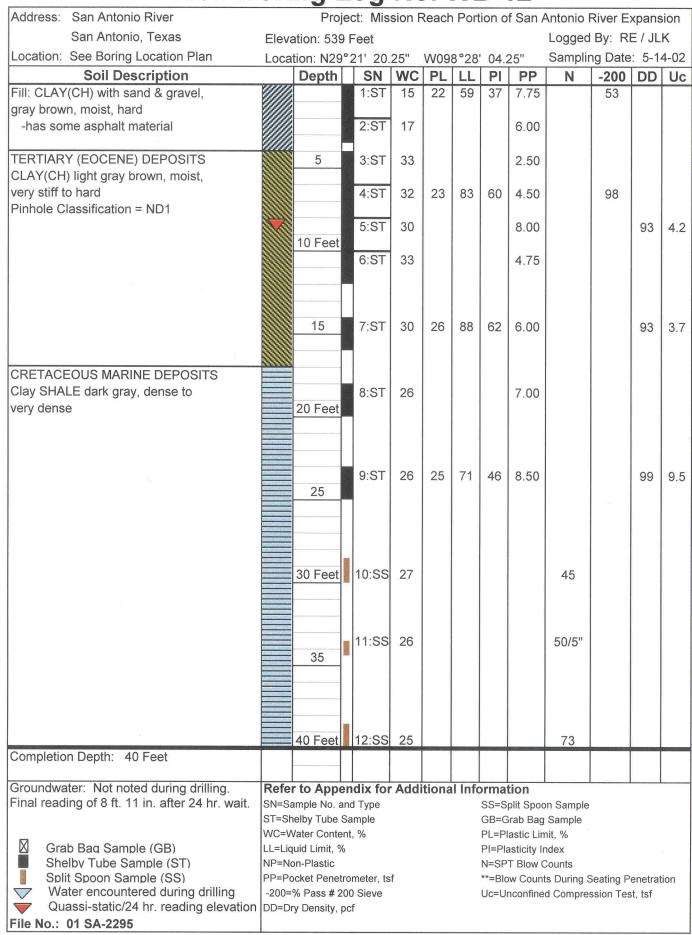




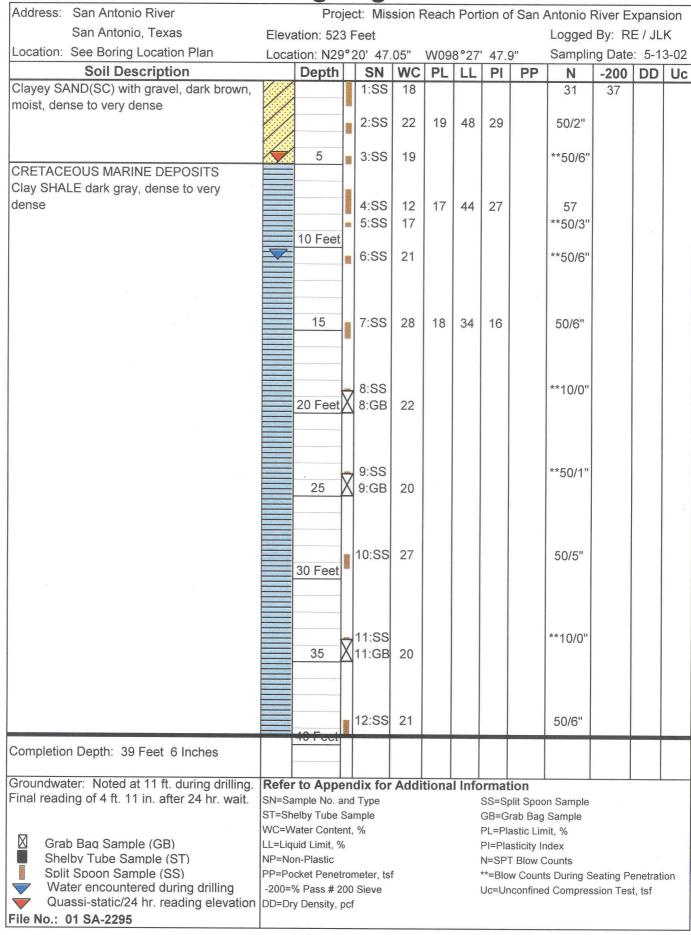


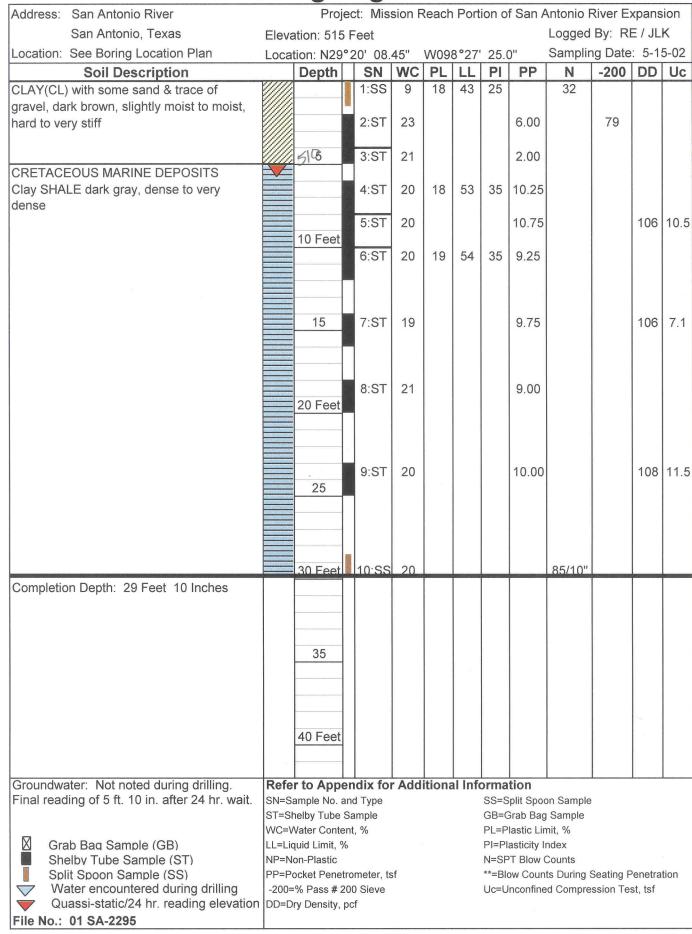


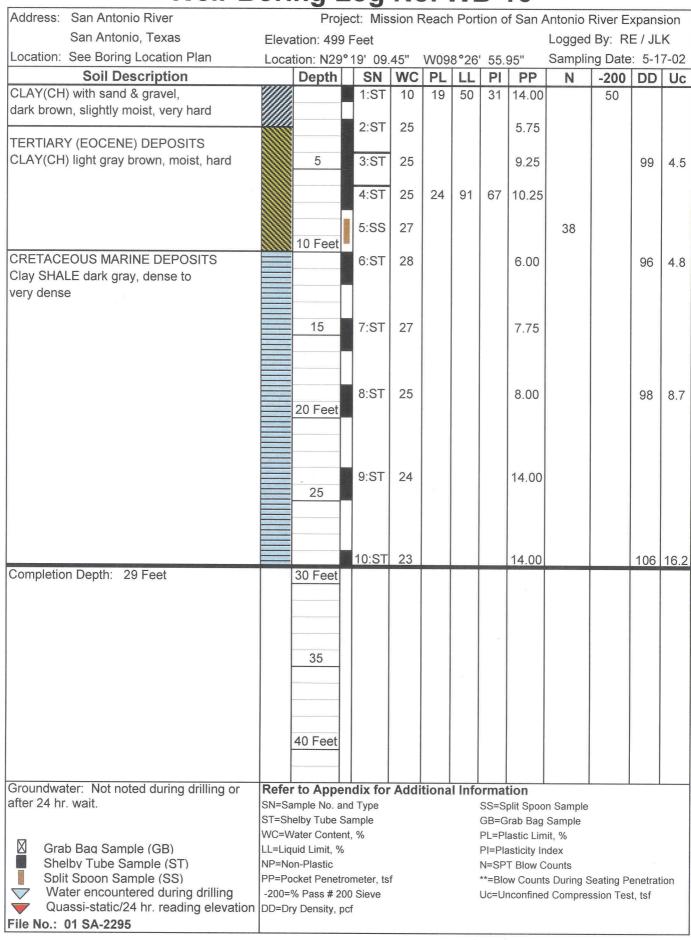


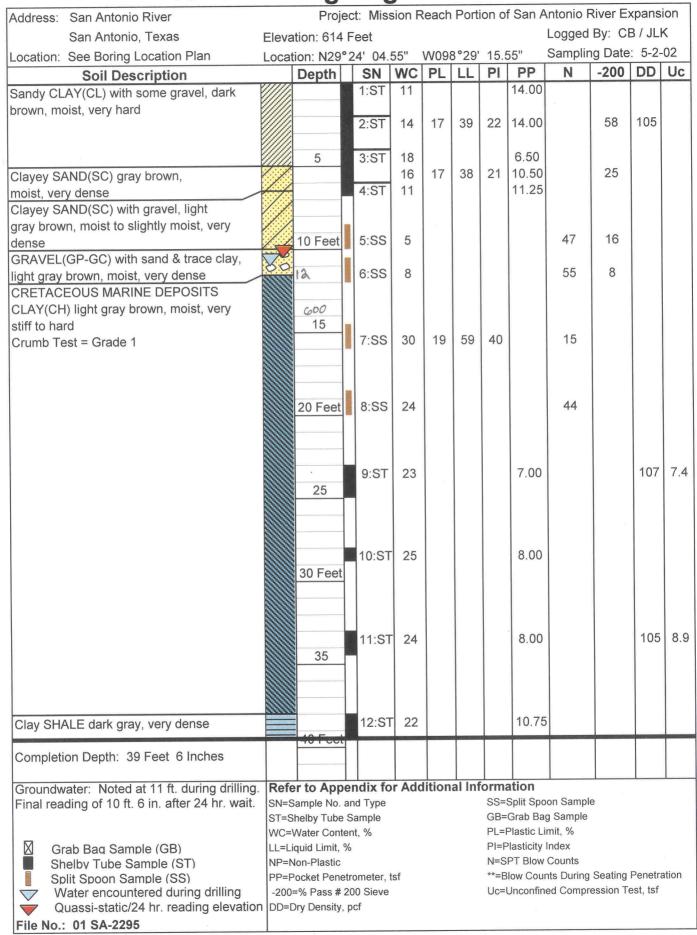


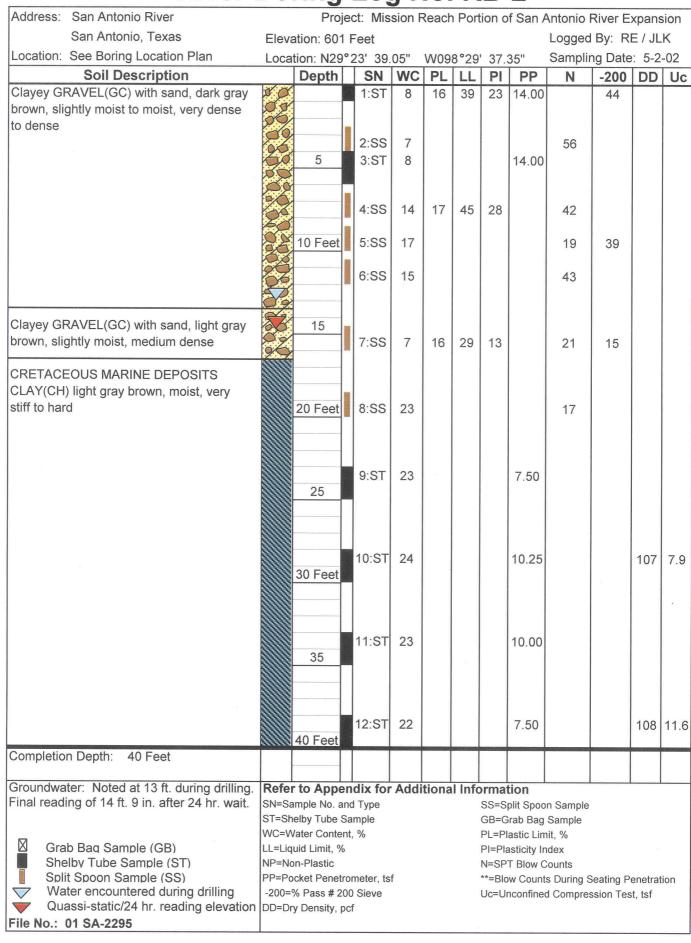
Address: San Antonio River		Pro								Antonio I	River E	ynang	sion
The state of the s					31011 F	react	1 - 00		ı Jalı /				
San Antonio, Texas		ation: 53					2 9 00 00			Logged	,		
Location: See Boring Location Plan	Loca	tion: N29	_	-		W09	7	Y		Samplin			_
Soil Description	1./	Depth		SN	WC	PL	LL	PI	PP	N	-200	DD	Uc
Clayey SAND(SC) with some gravel, dark				1:ST	7	21	48	27			36		
gray brown, slightly moist, medium dense	Z												
Clayey GRAVEL(GC) with sand, light gray brown, wet, loose	53			2:SS	26					_	40		
Clayey GRAVEL(GC) with sand,	66	5		2.33	20					5	40		
dark gray brown, moist, medium dense				3:SS	15					20	15		
Clayey GRAVEL(GC) with sand, light gray	33			0.00	10					20	15		
brown, wet, medium dense	22			4:SS	36	21	58	37		13	35		
TERTIARY (EOCENE) DEPOSITS		7						-					
CLAY(CH) light gray brown, moist,		10 Feet		5:SS	31					24			
very stiff to hard													
				6:SS	30	21	72	51		21			
									Ŷ.				
•		15					-						
				7:SS	29					24			
		20 Feet		8:SS	30	22	69	47		38			
		201000		0.00	30	22	03	7/		30			
								-					
		25		9:SS	27	-	^			48		98	8.2
		30 Feet		10:SS	23	21	72	51		52	1		
CRETACEOUS MARINE DEPOSITS				11:SS	25					FOICII		00	10
Clay SHALE dark gray, very dense		35		11.00	25					50/6"		99	13.4
Sia, Since dain gray, very delise		33											
				12:SS	24					50/6"			
		40 Feet				8 (A) (A) (A)			Z. A. C. Weinster				Properties (1)
Completion Depth: 39 Feet 6 Inches													
Occupation No. 1 1055	D .	4 .											
Groundwater: Noted at 3.5 ft. during drilling. Final reading of 2 ft. after 24 hr. wait.	1	r to App			Addi	tiona							
Final reading of 2 ft. after 24 ffr. wait.		ample No. a relby Tube		5.5						n Sample Sample			
		/ater Conte							astic Lin				
Grab Baq Sample (GB)		uid Limit, %		0					asticity In				
Shelby Tube Sample (ST)	100000 -07000	on-Plastic							T Blow C				
Split Spoon Sample (SS) Water encountered during drilling		cket Pene		statistical transfer for	f					ts During S	127		tion
Water encountered during drillingQuassi-static/24 hr. reading elevation		% Pass # 2 v Density						Uc=U	nconfine	d Compre	ssion Tes	st, tsf	
File No.: 01 SA-2295	100-01	, Denoity,	μĊ	u									
IIVII VI VI ALEVV													

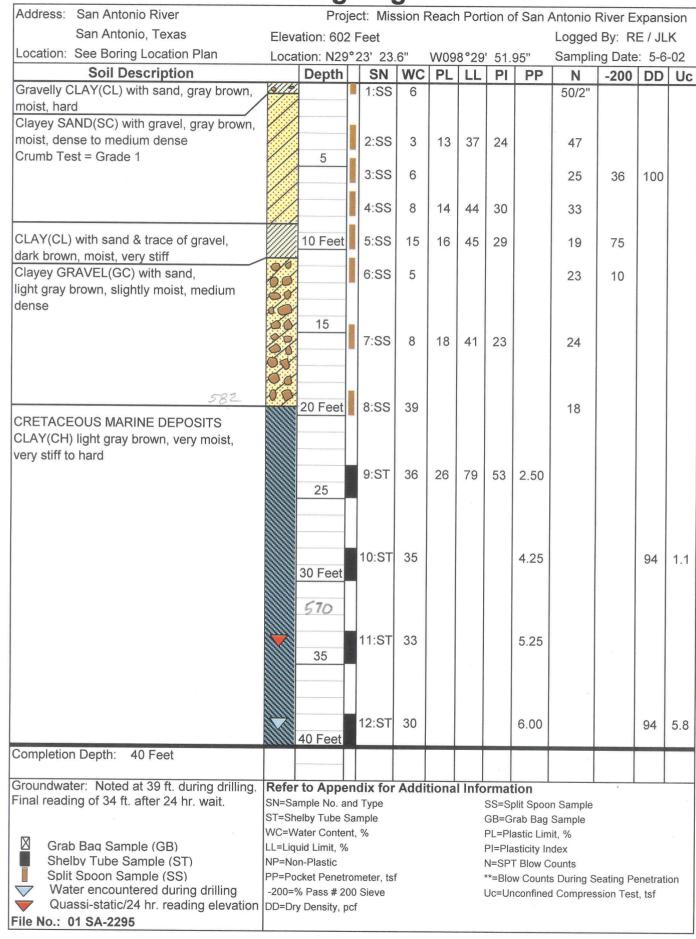


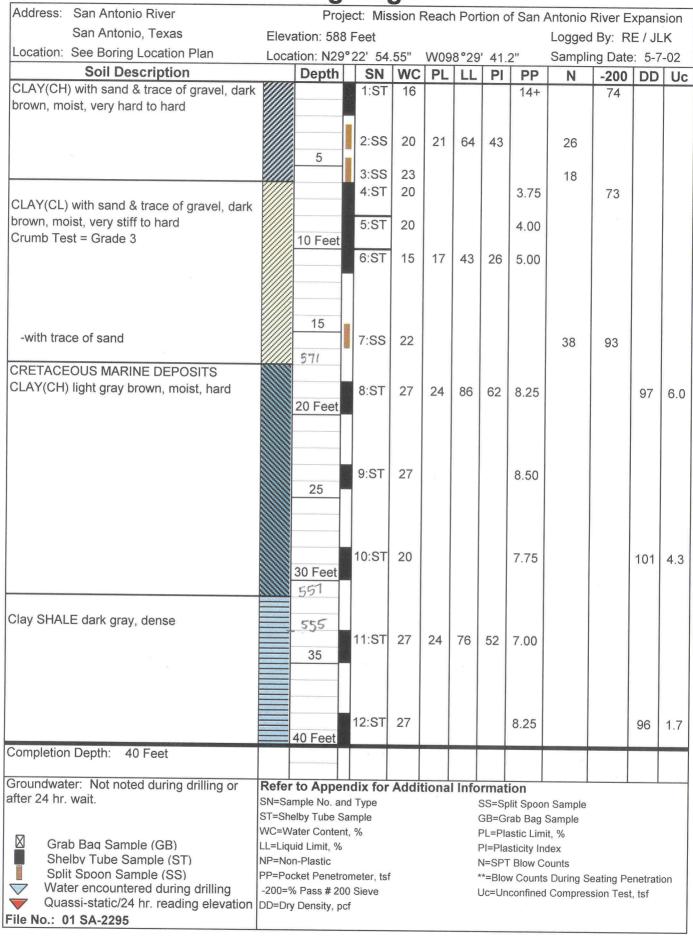


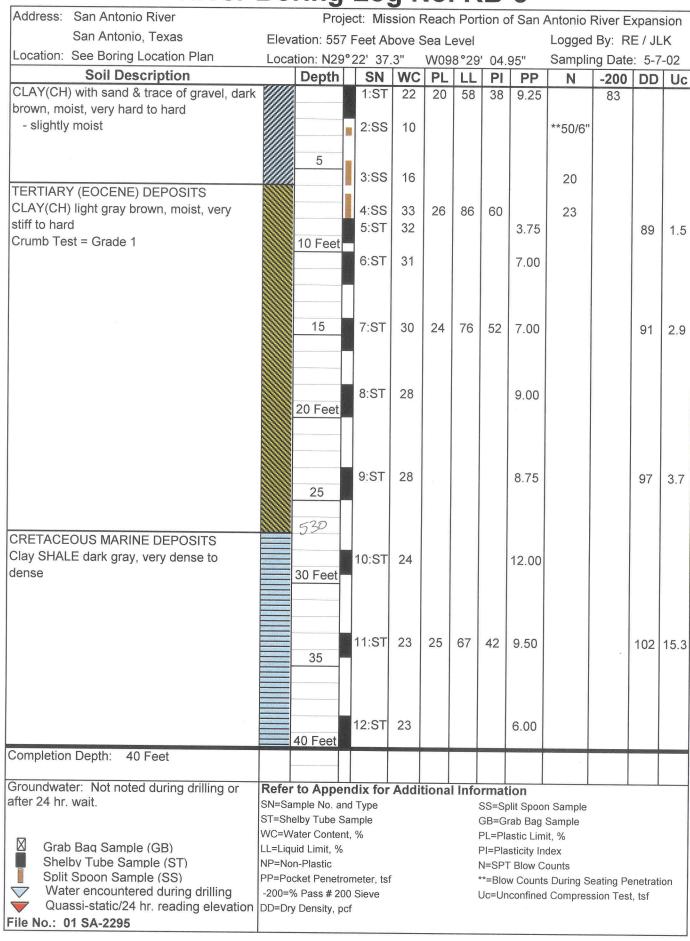


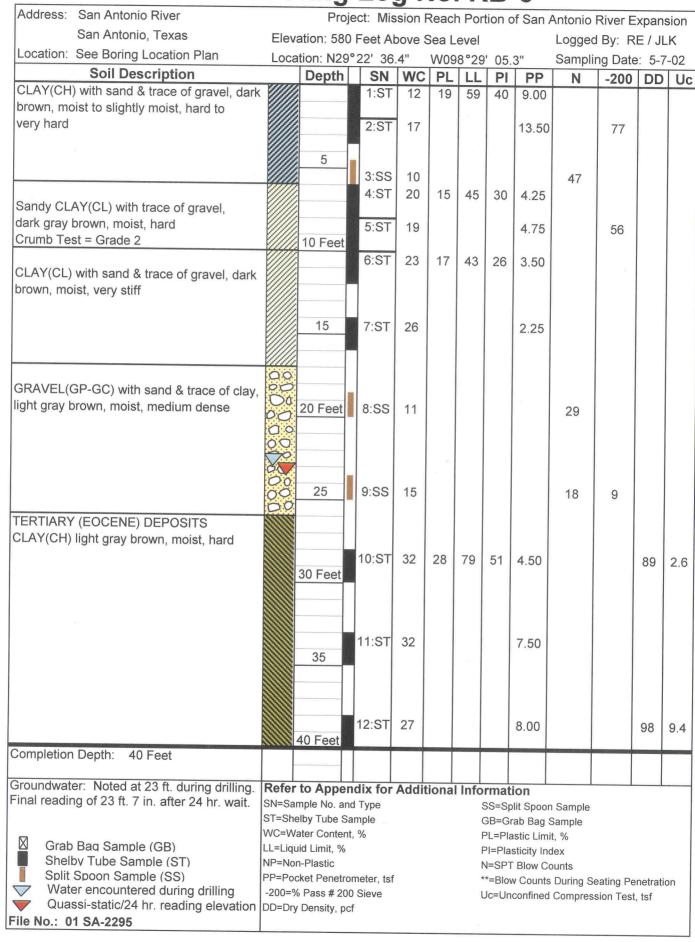


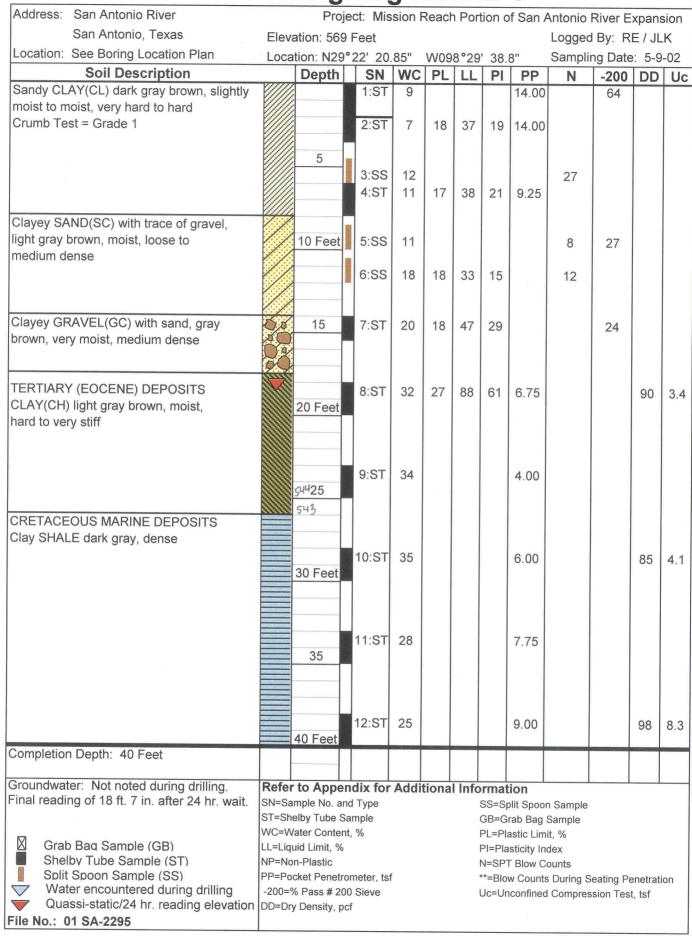


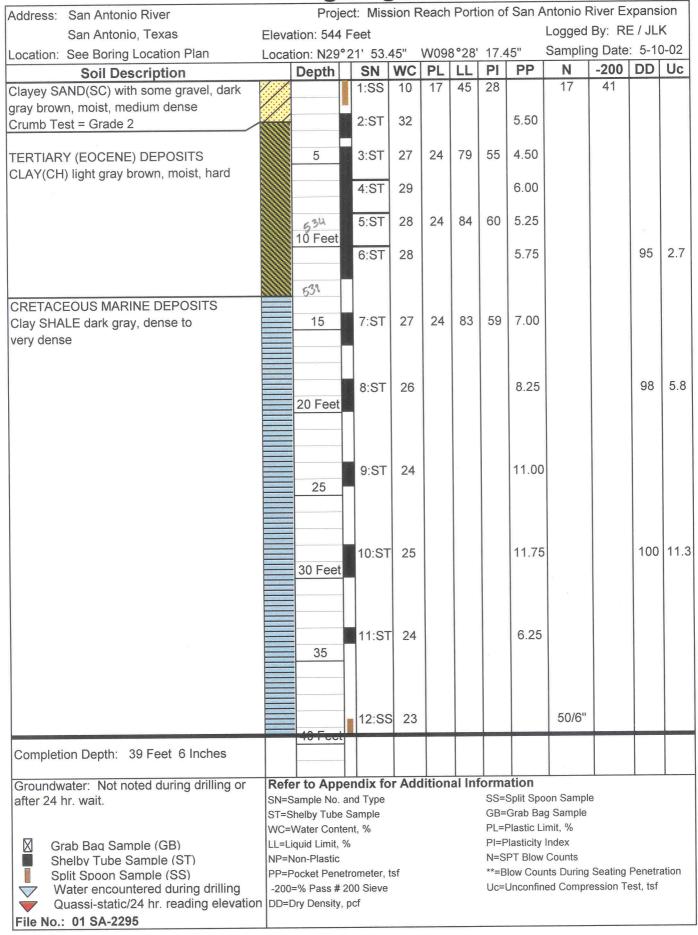


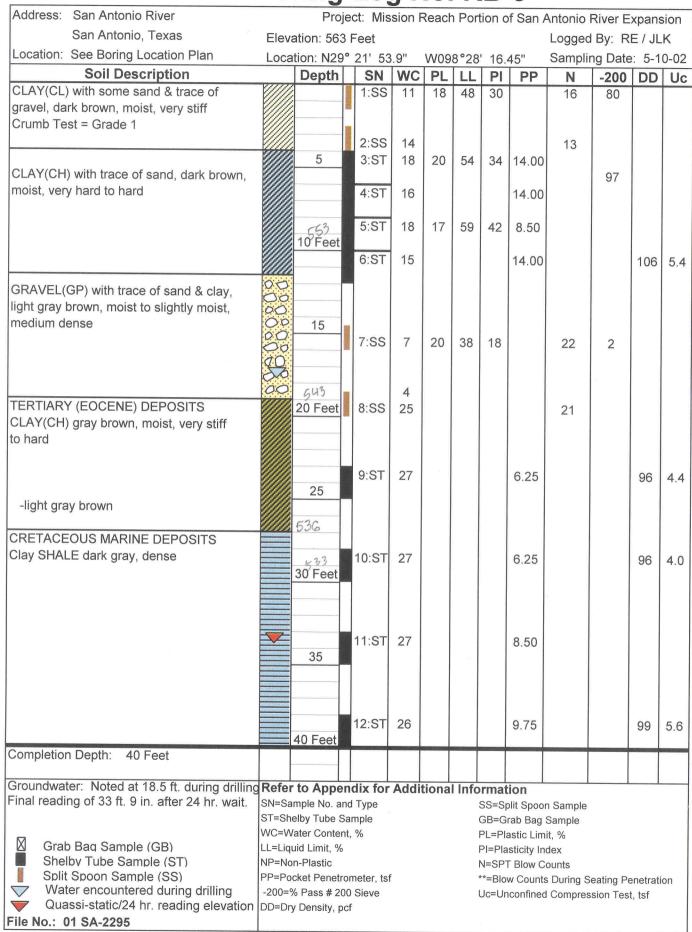


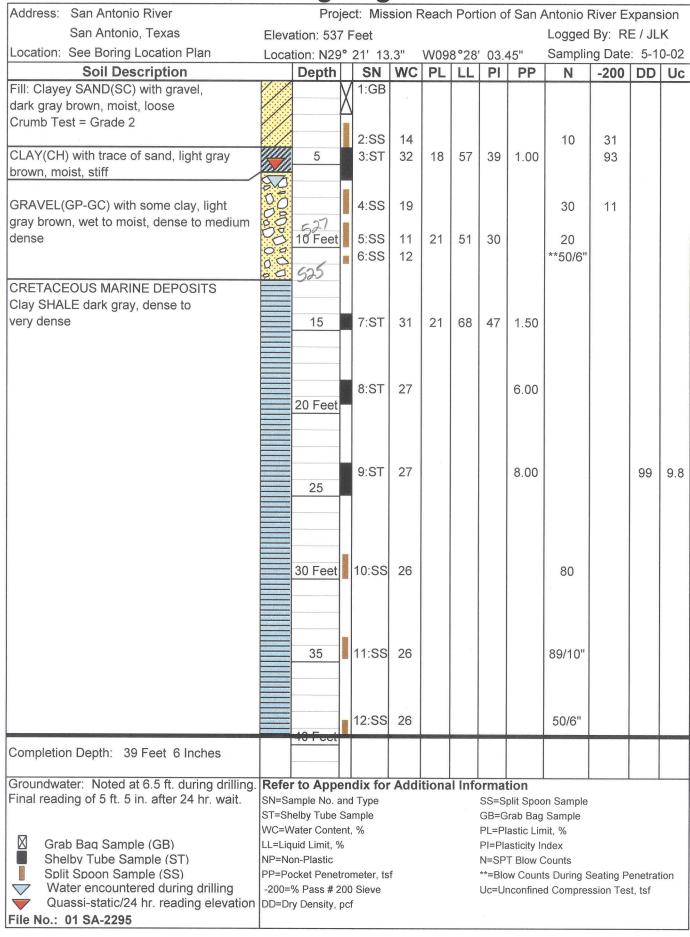




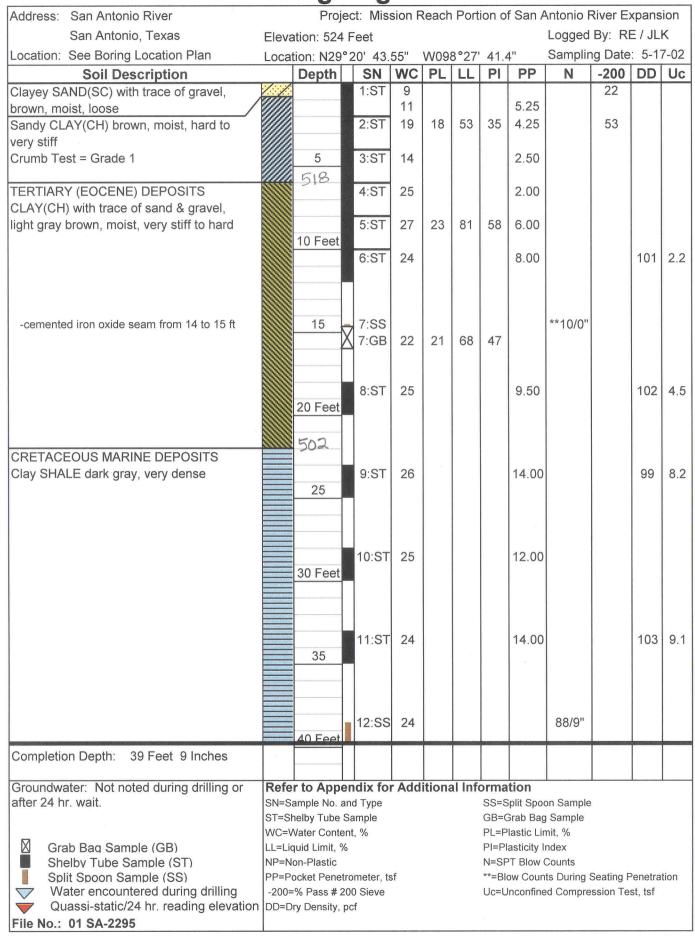


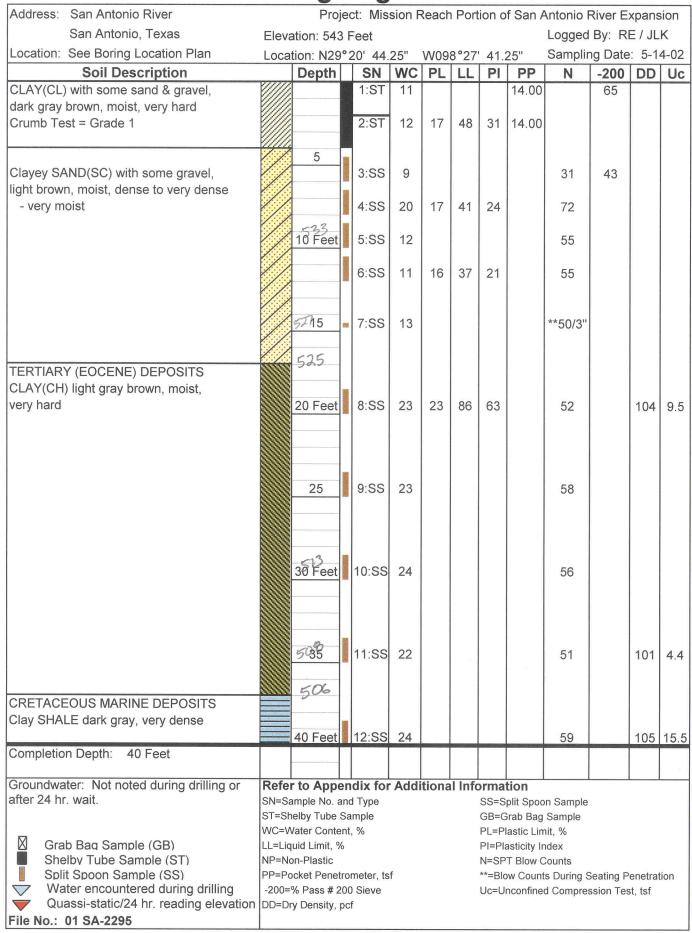


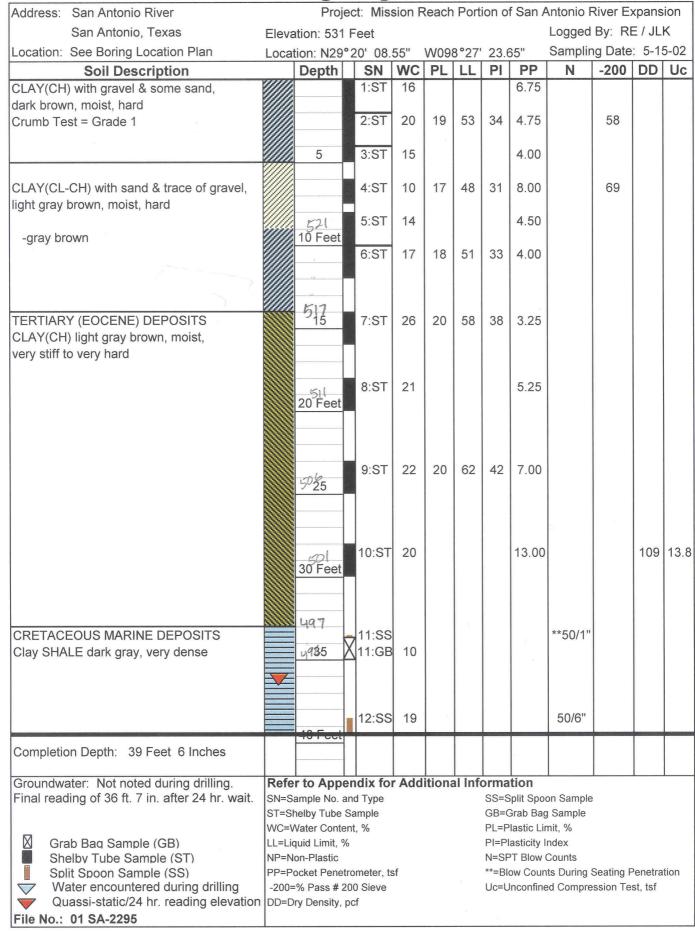


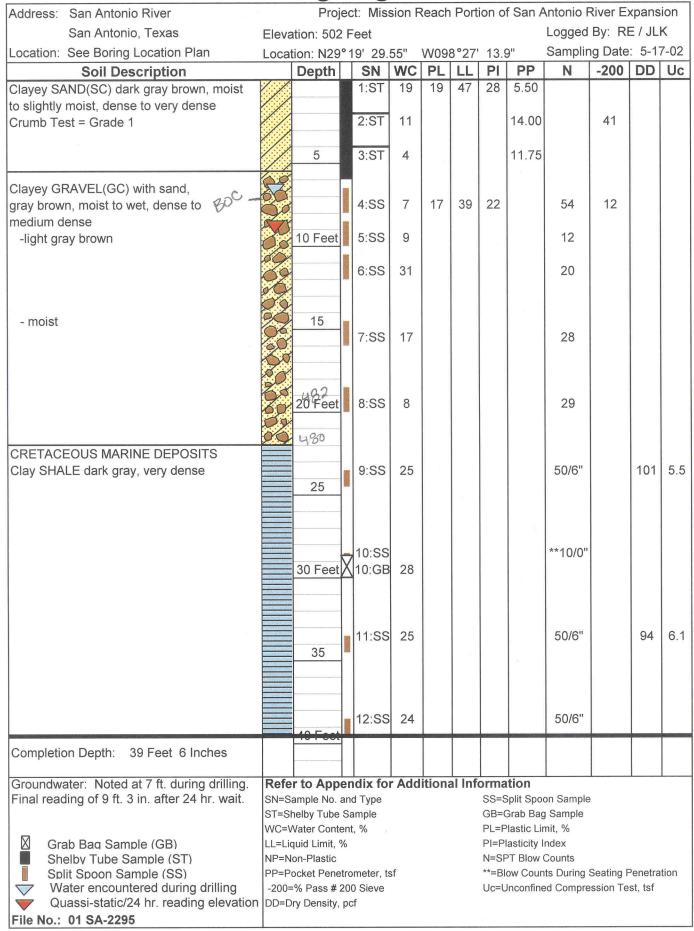


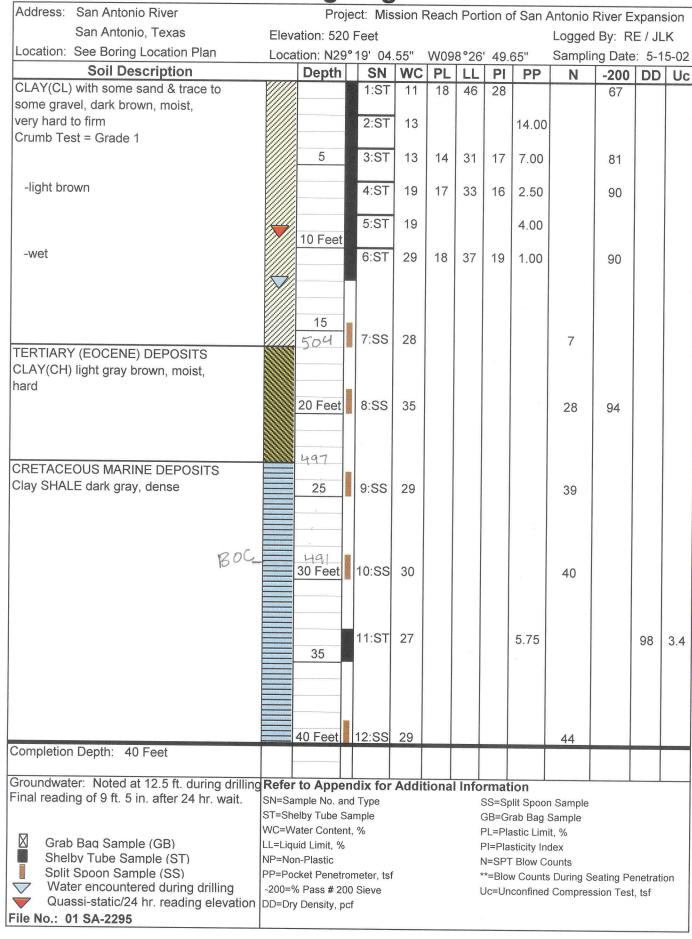
Address: San Antonio River			-	ct: Mis		2000	Par	ion o	of Con	Antonia	Divo- C	VEST	nio:-
San Antonio, Texas	Elov	etion: 55			51011	veact	ron	LIOIT C	n san /	Antonio Logged			
Location: See Boring Location Plan		tion: N29			6"	W09	0000	' 02	OII	Sampli			
Soil Description	LUCA	Depth	7	SN	wc	_	LL	PI	PP	N	-200		
CLAY(CH) with sand & trace to some		Ворит		1:SS	14		les les	1.		29	-200		00
gravel, moist, dark brown, hard to very hard Crumb Test = Grade 1				2:ST	14	18	50	32	14.00		70		
Sandy CLAY(CL) with trace of gravel, light gray brown, slightly moist, very hard		5		3:ST	9	17	33	16	11.75		57		
CLAY(CH-CL) with some sand, dark brown, moist, hard to very stiff				4:ST	14	4=			9.50				
dark brown, moist, hard to very stim		10 Feet	A COLUMN TO A COLU	5:ST 6:ST	18	17	53	36	9.00			103	9.9
										2			
		15		7:ST	24	17	46	29	2.00		81	90	0.7
Clayey GRAVEL(GC) with sand, light gray brown, moist, medium dense		530 20 Feet		8:SS	12					21	16		
GRAVEL(GP-GC) with trace clay, light gray brown, slighlty moist, medium dense	000000000000000000000000000000000000000	25 523		9:SS	2	-				21	5		
TERTIARY (EOCENE) DEPOSITS CLAY(CH) light gray brown, moist, hard		520 30 Feet		10:SS	23					41			
CRETACEOUS MARINE DEPOSITS Clay SHALE dark gray, very dense		518 35		11:SS	27				c	53			
		40 = 4		12:SS	26					50/6"			
Completion Depth: 39 Feet 6 Inches		101 000											
Groundwater: Noted at 16 ft. during drilling. Final reading of 16 ft. 6 in. after 24 hr. wait. Grab Baq Sample (GB) Shelby Tube Sample (ST) Split Spoon Sample (SS) Water encountered during drilling Quassi-static/24 hr. reading elevation File No.: 01 SA-2295	SN=Sample No. and Type SS=Split Spoon Sample ST=Shelby Tube Sample WC=Water Content, % LL=Liquid Limit, % NP=Non-Plastic PP=Pocket Penetrometer, tsf -200=% Pass # 200 Sieve SS=Split Spoon Sample GB=Grab Bag Sample PL=Plastic Limit, % PI=Plasticity Index N=SPT Blow Counts **=Blow Counts During Seating Penetration Uc=Unconfined Compression Test, tsf						ion						

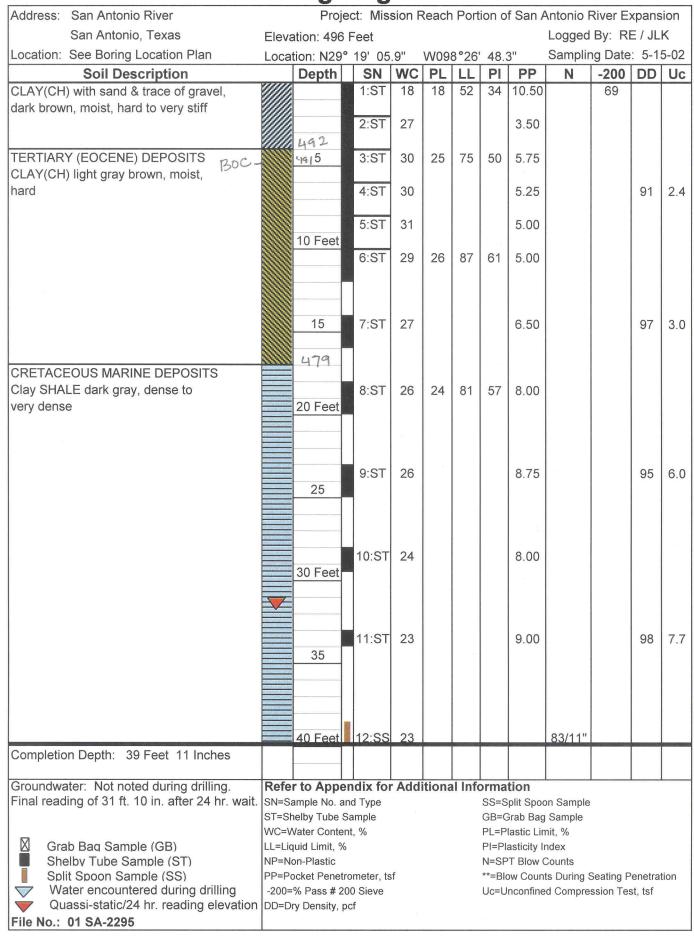












KEY TO CLASSIFICATION SYMBOLS USED ON BORING LOGS

MA	MAJOR DIVISIONS GROUP SYMBOL			DESCRIPTIONS	
Size	ction e Size	Gravels o Fines)	GW		Well-Graded Gravels, Gravel-Sand Mixtures, Little or no Fines
30 Sieve	/ELS Coarse Fa No. 4 Siev	Clean Gravels (Little or no Fines)	GP	0000	Poorly-Graded Gravels, Gravel-Sand Mixtures, Little or no Fines
SOILS	GRAVELS More Than Half of Coarse Faction is LARGER Than No. 4 Sieve Size	ravels With Fines Appreciable Amount of Fines)	GM		Silty Gravels, Gravel-Sand-Silt Mixtures
Ė	More Thai	Gravels With Fines (Appreciable Amount of Fines)	GC		Clayey Gravels, Gravel-Sand-Clay Mixtures
COARSE-GRAINED of Material LARGER	raction ve Size	Sands to Fines)	sw		Well-Graded Sands, Gravelly Sands, Little or no Fines
COAR alf of Mai	SANDS Iff of Coarse Fi	Clean Sands (Little or no Fines)	SP		Poorly-Graded Sands, Gravelly Sands, Little or no Fines
e Than H	SANDS More Than Half of Coarse Fraction is SMALLER Than No. 4 Sieve Size	Sands With Fines Appreciable Amount of Fines)	SM		Silty Sands, Sand-Silt Mixtures
More	More Th	Sands With (Appreciable Amount of Fi	sc		Clayey Sands, Sand-Clay Mixtures
SOILS SMALLER ize.	∞ o o ≻	Liquid Limit Less Than 50	ML		Inorganic Silts & Very Fine Sands, Rock Flour, Silty or Clayey Fine Sands or Clayey Silts with Slight Plasticty
Meterial is SM Sieve Size.	SILTS & CLAYS	Liquio Less 5	CL		Inorganic Clays of Low to Medium Plasticity, Gravelly Clays, Sandy Clays, Silty Clays, Lean Clays
	% S	Liquid Limit Greater Than 50	MH		Inorganic Silts, Micaceous or Diatomaceous Fine Sand or Silty Soils, Elastic Silts
FINE-GRA More Than Half of Than No. 200	SILTS & CLAYS	Liquid Greate 5	СН		Inorganic Clays of High Plasticity, Fat Clays
	SANDS	STONE			Massive Sandstones, Sandstones with Gravel Clasts
	MARL	STONE			Indurated Argillaceous Limestones
ONAL	LIMES	TONE		1000 1000	Massive or Weakly Bedded Limestones
ORMATIONAL MATERIALS	CLAYS	STONE			Mudstone or Massive Claystones
ш		OW MARINE			Tertiary Clay Deposits
	MARIN	IE CLAYS			Cretaceous Clay Deposits
	CROU	NDWATER		V	Indicates Final Observed Groundwater Level

APPENDIX

Laboratory and Field Test Procedures

Soil Classification Per ASTM D2487-93

This soil testing standard was used for classifying soils according to the Unified Soil Classification System. The soil classifications of the earth materials encountered are as noted in the attached boring logs.

Soil Water Content Per ASTM D2216-92

This test determines the water content of soil or rock expressed as a percentage of the solid mass of the soil. The test results are listed under **MC** in the attached boring logs.

Soil Liquid Limit Per ASTM D4318-93

The soil Liquid Limit identifies the upper limit soil water content at which the soil changes from a moldable (plastic) physical state to a liquid state. The Liquid Limit water content is expressed as a percentage of the solid mass of the soil. The test results are listed under **LL** in the attached boring logs.

Soil Plastic Limit Per ASTM D4318-93

The soil Plastic Limit identifies a lower limit soil water content at which the soil changes from a moldable (plastic) physical state to a non-moldable (semi-solid) physical state. The Plastic Limit water content is expressed as a percentage of the solid mass of the soil. The test results are listed under **PL** in the attached boring logs.

Plasticity Index Per ASTM D4318-93

This is the numeric difference between the Liquid Limit and Plastic Limit. This index also defines the range of water content over which the soil-water system acts as a moldable (plastic) material. Higher Plasticity Index (PI) values indicate that the soil has a greater ability to change in soil volume or shrink and swell with lower or higher water contents, respectively. The test results are listed under PI in the attached boring logs.

Standard Penetration Test (SPT) and Split Spoon Sampler (SS) per ASTM D 1586

This is the standard test method for both the penetration test and split-barrel (spoon) sampling of soils. This sampling method is used for soils or rock too hard for sampling using Shelby Tubes. The method involves penetration of a split spoon sampler into the soil or rock through successive blows of a 140 pound hammer in a prescribed manner.

Blow Counts (N) per ASTM D 1586

This is the number of blows required to drive a Split Spoon Sampler by means of a 140 pound hammer for a distance of 12 inches in accordance with the variables stated in the test procedures.

Shelby Tube (ST) per ASTM D 1587

This procedure is for using a thin-walled metal tube to recover relatively undisturbed soil samples suitable for laboratory tests of physical properties.

Rock Core per ASTM D 2113

This procedure is for using diamond core drilling equipment to obtain core samples of rock and some soils that are too hard to sample by soil-sampling methods.

Dry Density (DD) per ASTM D 2937

This procedure is for the determination of in-place density of soil. The test results are measured in pounds per cubic foot, pcf.

Unconfined Compression Test (UC) per ASTM D 2166

This test method covers the determination of the unconfined compressive strength of cohesive soil in the undisturbed, remolded, or compacted condition, using strain-controlled application of the axial load.

Minus No. 200 Sieve per ASTM D 1140

This test method covers determination of the amount of material finer than a Number 200 sieve by washing. The results are stated as a percent of the total dry weight of the sample.

Pocket Penetrometer (PP): This test method is an accepted modification of ASTM D 1558 test method for establishing the moisture-penetration resistance relationships of fine-grained soils. The test results are measured in tons per square foot, tsf. The strength values provided by this method should be considered qualitatively.

Rock Quality Designation (RQD): The measure of the quality of a rock mass defined by adding intact rock core pieces greater than four inches in length by the total length of core advance per ASTM 6032.

Recovery Ratio (REC): The Recovery Ratio is equal to the total length of core recovered divided by the total length of core advance.

Boring Logs: This is a summary of the above described information at each boring location.